DESIGN AND CONSTRUCTION GROUP
THE GOVERNOR NELSON A. ROCKEFELLER
EMPIRE STATE PLAZA
ALBANY, NY 12242

ADDENDUM NO. 1 TO PROJECT NO. 44854

CONSTRUCTION, HVAC WORK, PLUMBING WORK, ELECTRICAL WORK
PROVIDE CENTRAL SECURITY UNIT, BUILDING No. 51
HIGHLAND RESIDENTIAL CENTER
629 NORTH CHODIKEE LAKE ROAD
HIGHLAND, NY 12528

February 1, 2016

NOTE: This Addendum forms a part of the Contract Documents. Insert it in the Project Manual. Acknowledge receipt of this Addendum in the space provided on the Bid Form.

STRUCTURAL DRAWINGS

1. Addendum Drawings:
   a. Drawing No. S-100 noted "ADDENDUM DRAWING 01/29/2016"accompanies this Addendum and forms part of the Contract Documents.

FIRE ALARM DRAWINGS

2. Drawing FA-001, General Notes: Delete note Q in its entirety.

END OF ADDENDUM

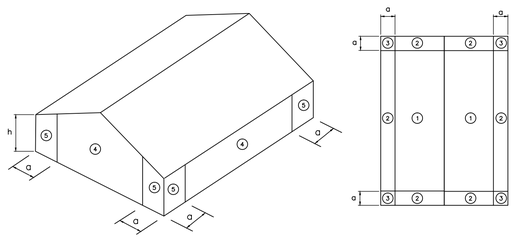
Margaret F. Larkin
Executive Director
Design and Construction

GENERAL NOTES:

1. REFER TO THE PROJECT MANUAL FOR GOVERNING JOB REQUIREMENTS AND MATERIAL SPECIFICATIONS. THE FOLLOWING NOTES ARE SUPPLEMENTAL TO THE ABOVE REQUIREMENTS.
2. ALL DIMENSIONS TO, OF, AND IN EXISTING STRUCTURES SHALL BE VERIFIED IN FIELD BY CONTRACTOR WITH ALL DISCREPANCIES REPORTED TO THE ENGINEER.
3. DO NOT CHANGE THE SIZE NOR SPACING OF STRUCTURAL ELEMENTS WITHOUT THE APPROVAL OF THE ENGINEER.
4. DETAILS SHOWN ARE TYPICAL AND APPLY TO SIMILAR CONDITIONS UNLESS NOTED OTHERWISE.
5. THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.
6. CONTRACTOR SHALL BRACE BUILDING AS REQUIRED FOR CONSTRUCTION AND WIND LOADS UNTIL ALL STRUCTURAL ELEMENTS NEEDED FOR STABILITY ARE INSTALLED. THESE ELEMENTS ARE AS FOLLOWS: ROOF DECK, BRACING MEMBERS, SHEAR WALLS, ETC.
7. THE DESIGN IS BASED ON THE BUILDING CODE OF NEW YORK STATE, 2010 EDITION.
8. CONTRACTOR SHALL DETERMINE EXACT LOCATION OF EXISTING UTILITIES BEFORE COMMENCING WORK. CONTRACTOR AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY FAILURE TO EXACTLY LOCATE AND PRESERVE UNDERGROUND UTILITIES.
9. INCORRECTLY FABRICATED, DAMAGED, OR OTHERWISE MISFITTING OR NONCONFORMING MATERIALS OR CONDITIONS SHALL BE REPORTED TO THE ENGINEER PRIOR TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER 'S APPROVAL.
10. EACH CONTRACTOR SHALL COOPERATE WITH THE DIRECTOR'S REPRESENTATIVE, AND COORDINATE WORK WITH THE WORK OF OTHERS.
11. VERIFY SIZE AND LOCATION OF OPENINGS PRIOR TO BEGINNING WORK. FOR DIMENSIONS NOT SHOWN, SEE MECHANICAL, ELECTRICAL, CIVIL AND ARCHITECTURAL DRAWINGS.
12. VERIFY SIZE AND LOCATION OF EQUIPMENT PADS WITH MECHANICAL AND/OR ELECTRICAL CONTRACTOR AND EQUIPMENT MANUFACTURER.

DESIGN DATA:

1. FLOOR LIVE LOADS
   - A. ASSEMBLY AREAS: 100 PSF
   - B. LIGHT STORAGE AREAS: 125 PSF
   - C. OFFICE SPACE: 50 PSF
   - D. ALL OTHER AREAS: 100 PSF
2. ROOF SNOW LOAD
   - A. GROUND SNOW LOAD: 40 PSF
   - B. FLAT-ROOF SNOW LOAD: 31 PSF
   - C. SNOW EXPOSURE FACTOR: 1.0
   - D. SNOW LOAD IMPORTANCE FACTOR: 1.1
   - E. THERMAL FACTOR: 1.0
3. WIND DESIGN DATA
   - A. BASIC WIND SPEED (3-SECOND GUST): 90 MPH
   - B. WIND IMPORTANCE FACTOR: 1.15
   - C. WIND EXPOSURE CATEGORY: B
   - D. INTERNAL PRESSURE COEFFICIENTS: ±0.18
4. EARTHQUAKE DESIGN DATA
   - A. SEISMIC IMPORTANCE FACTOR: 1.25
   - B. MAPPED SPECTRAL RESPONSE ACCELERATIONS:
     - Ss: 0.18g
     - S1: 0.04g
   - C. SITE CLASS: C
   - D. SEISMIC DESIGN CATEGORY: A
   - E. DESIGN BASE SHEAR: 0.0132xW (KIPS)
   - F. BASIC SEISMIC FORCE RESISTING SYSTEM: ORDINARY REINFORCED MASONRY SHEAR WALLS
   - G. RESPONSE MODIFICATION FACTOR: R=2
   - H. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE METHOD
5. DEAD LOADS:
   - A. ROOF LOAD:
     - METAL STANDING SEAM ROOFING: 3 PSF
     - PLYWOOD SHEATHING: 2 PSF
     - PRE-ENGINEERED WOOD TRUSSES: 8 PSF
     - INSULATION: 2 PSF
     - GYPSUM CEILING: 6 PSF
     - MECH & ELEC: 5 PSF
     - MISC.: 2 PSF
     - TOTAL 28.2 PSF
6. SOIL BEARING
   - A. FOUNDATION DESIGN BASED ON PRESUMPTIVE SOIL BEARING CAPACITY OF 6,400 PSF ON UNDISTURBED MATERIAL OR COMPACTED FILL. REFER TO PROJECT MANUAL FOR GEOTECHNICAL REPORT PREPARED BY C&S COMPANIES DATED JULY 10, 2015
   - B. BEARING STRATUM SHALL BE FIELD VERIFIED BY GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF ANY MATERIAL.
7. COMPONENTS AND CLADDING LOAD DIAGRAM
   - A. BASIC WIND PRESSURE: 14.20 PSF
   - B. EDGE STRIP DIMENSION, a: 10% OF LEAST HORIZONTAL DIMENSION OR 0.4h, WHICHEVER IS SMALLER, BUT NOT LESS THAN EITHER 4% OF LEAST HORIZONTAL DIMENSION OR 3 FT.
   - C. COMPONENT AND CLADDING PRESSURES:
     - ZONE 1 −15.34 PSF
     - ZONE 2 −18.18 PSF
     - ZONE 3 −18.18 PSF
     - ZONE 4 −15.70 PSF
     - ZONE 5 −18.60 PSF



EARTHWORK:

1. SUBBASE MATERIAL BELOW FLOOR SLAB:
   NATURALLY OR ARTIFICIALLY GRADED MIXTURE OF NATURAL OR CRUSHED GRAVEL, CRUSHED STONE, AND NATURAL OR CRUSHED SAND, ASTM S2940, WITH AT LEAST 95 PERCENT PASSING A 1 1/2-INCH SIEVE AND NOT MORE THAN 8 PERCENT PASSING A NO. 200 SIEVE.
2. BACKFILL AND FILL MATERIALS:
   ASTM D2487 SOIL CLASSIFICATION GROUPS GW, GP, GM, SW, SP, AND SM; FREE OF ROCK OR GRAVEL LARGER THAN 2 INCHES IN ANY DIMENSION, DEBRIS, WASTE, FROZEN MATERIALS, VEGETATION AND OTHER DELETERIOUS MATTER.
3. COMPACTION
   - A. PLACE BACKFILL AND FILL MATERIALS IN LAYERS NOT MORE THAN 8 INCHES IN LOOSE DEPTH FOR MATERIAL COMPACTED BY HEAVY EQUIPMENT, AND NOT MORE THAN 4 INCHES IN LOOSE DEPTH FOR MATERIAL COMPACTED BY HAND-OPERATED TAMPERS.
   - B. PLACE BACKFILL AND FILL MATERIALS EVENLY ON ALL SIDES OF STRUCTURES TO REACH REQUIRED ELEVATIONS. PLACE BACKFILL AND FILL UNIFORMLY ALONG THE FULL LENGTH OF EACH STRUCTURE.
   - C. COMPACT THE TOP 12 INCHES BELOW SUBGRADE AND EACH LAYER OF BACKFILL OR FILL MATERIAL AND THE SUBBASE TO A MINIMUM OF 95 PERCENT MAXIMUM DRY DENSITY ACCORDING TO ASTM D1557.
   - D. NO BACKFILLING OR COMPACTION SHALL TAKE PLACE AGAINST ANY CAST-IN-PLACE CONCRETE FOOTINGS OR SLABS PRIOR TO 7 DAYS INITIAL CONCRETE SET, OR AGAINST ANY CAST-IN-PLACE CONCRETE WALLS PRIOR TO ACHIEVING 75% COMPRESSIVE STRENGTH, 0.75 F'C.
   - E. HEAVY EQUIPMENT SHALL NOT BE OPERATED WITHIN 4 FEET OF ANY STRUCTURE. HEAVY VIBRATORY COMPACTORS SHALL NOT BE OPERATED WITHIN 4 FEET OF ANY STRUCTURE.
   - F. COMPACTION TESTING SHALL BE PERFORMED TO ASCERTAIN THE COMPACTED DENSITY OF THE FILL AND BACKFILL MATERIALS IN ACCORDANCE WITH THE FOLLOWING METHODS:

     IN-PLACE OF RELATIVE DENSITY:

     1. METHOD: AASHTO T191, SAND CONE METHOD
        AASHTO T238, NUCLEAR METHOD
     2. NUMBER OF TESTS: ONE (1) PER 8" VERTICAL LIFT
   - G. THE DIRECTOR'S REPRESENTATIVE MAY DIRECT ADDITIONAL TESTS TO ESTABLISH GRADATION, MAXIMUM DENSITY, AND IN-PLACE DENSITY AS REQUIRED BY WORKING CONDITIONS, AT THE CONTRACTOR'S EXPENSE.
   - H. ACCEPTANCE CRITERIA: THE SOLE CRITERION FOR ACCEPTABILITY OF IN-PLACE FILL SHALL BE IN SITU DRY DENSITY. MINIMUM DRY DENSITY FOR ALL FILL OR BACKFILL SHALL BE 95 PERCENT OF THE MAXIMUM DRY DENSITY. IF A TEST FAILS TO QUALIFY, THE FILL SHALL BE FURTHER COMPACTED AND RE-TESTED. SUBSEQUENT TEST FAILURES SHALL BE FOLLOWED BY REMOVAL AND REPLACEMENT OF THE MATERIAL.

FOUNDATION NOTES:

1. BEAR ALL FOOTINGS ON MINIMUM 6" COMPACTED CRUSHED STONE. THE SOUTHEAST CORNER OF THE BUILDING FOUNDATION WILL BEAR DIRECTLY ON ROCK. REFER TO S-100 FOR EXACT LOCATION.
2. FOOTINGS HAVE BEEN DESIGNED FOR A SOIL BEARING PRESSURE AS INDICATED IN THE DESIGN DATA. BEARING STRATUM FOR THIS CAPACITY SHALL BE VERIFIED IN FIELD BY DIRECTOR'S REPRESENTATIVE BEFORE PLACING CONCRETE FOOTINGS.
3. SOIL BEARING SURFACES, PREVIOUSLY ACCEPTED BY DIRECTOR'S REPRESENTATIVE, WHICH ARE ALLOWED TO BECOME SATURATED, FROZEN, OR DISTURBED SHALL BE REWORKED TO SATISFACTION OF DIRECTOR'S REPRESENTATIVE.
4. DO NOT PLACE FOOTINGS IN WATER OR ON FROZEN GROUND.
5. DO NOT ALLOW GROUND BENEATH FOOTINGS TO FREEZE.
6. CENTER FOOTINGS UNDER WALLS, PIERS, OR COLUMNS UNLESS NOTED OTHERWISE.
7. VERIFY SIZE AND LOCATION OF OPENINGS PRIOR TO BEGINNING WORK. FOR DIMENSIONS NOT SHOWN, SEE ARCH. DWGS.

CAST-IN-PLACE CONCRETE NOTES:

1. ALL DOWELS, ANCHOR BOLTS, EMBEDDED STEEL, ELECTRICAL CONDUITS, PIPE SLEEVES, PIPING, INSERTS, GROUNDS, AND ALL OTHER EMBEDDED ITEMS AND FORMED DETAILS SHALL BE IN PLACE BEFORE START OF CONCRETE PLACEMENT. FOR EMBEDDED ITEMS AND REQUIRED DETAILS, SEE MECHANICAL, ELECTRICAL, AND ARCHITECTURAL DRAWINGS. VERIFY SIZE AND LOCATION OF ALL OPENINGS.
2. CONCRETE, UNLESS NOTED OTHERWISE, SHALL BE NORMAL WEIGHT, AIR ENTRAINED AND HAVE THE FOLLOWING MINIMUM 28-DAY COMPRESSIVE STRENGTHS:
   1. ALL CONCRETE: F'C = 4,000 PSI
3. ALL PIPING PENETRATIONS THROUGH NEW STRUCTURAL SLABS ARE TO BE SLEEVED OR CHASED. NO CORING OF SLAB IS PERMITTED.
4. REINFORCE ALL CONCRETE ELEMENTS (FOOTINGS, WALLS, PIERS, AND SLABS). REINFORCEMENT SHOWN PERTAINS TO ALL TYPICAL CONDITIONS.
5. SPLICES IN REINFORCEMENT SHALL MEET CLASS B TENSION LAP REQUIREMENTS UNLESS NOTED OTHERWISE.
6. REINFORCEMENT SHALL BE COLD BENT WHENEVER BENDING IS REQUIRED.
7. PROVIDE CORNER BARS IN FOOTINGS, THE SAME SIZE AND NUMBER AS CONTINUOUS REINFORCEMENT.
8. WHERE REQUIRED, STEP FOOTINGS UP OR DOWN IN RATIO OF TWO HORIZONTALS TO ONE VERTICAL. CAST STEPPED FOOTINGS MONOLITHICALLY.
9. DOWEL CONCRETE WALLS AND PIERS INTO FOOTINGS WITH DOWELS THE SAME SIZE AND SPACING AS VERTICAL REINFORCEMENT. EXTEND DOWELS TO WITHIN 3" OF BOTTOM OF FOOTING, TERMINATED WITH ACI STD. 90 DEGREE HOOK, UNLESS NOTED OTHERWISE.
10. PROVIDE KEYS IN CONCRETE WALLS, PIERS AND FOOTINGS AT INTERSECTION OF CONCRETE.
11. PROVIDE 3/4" X 3/4" CHAMFER AT ALL EXPOSED CORNERS UNLESS NOTED OTHERWISE.
12. NO HOLES OR OPENINGS ARE PERMITTED THROUGH CONCRETE SLABS, BEAMS, OR WALLS EXCEPT AS FOLLOWS:
    - A. WHERE SHOWN AND AS DETAILED ON DRAWINGS.
    - B. MISCELLANEOUS HOLES THROUGH SLABS OR WALLS WHICH DO NOT DISPLACE MORE THAN ONE BAR. THESE DO NOT REQUIRE ADDITIONAL REINFORCEMENT.
13. LOCATE ADDITIONAL CONSTRUCTION JOINTS REQUIRED TO FACILITATE CONSTRUCTION AS ACCEPTABLE TO ENGINEER. PLACE REINFORCEMENT CONTINUOUSLY THROUGH JOINT. DETAIL JOINT ON SHOP DRAWINGS.
14. CAST CONCRETE ON SLOPED SURFACES BEGINNING AT LOWEST ELEVATION AND CONTINUING MONOLITHICALLY TOWARD HIGHER ELEVATIONS UNTIL INTENDED POUR IS COMPLETED.

SLAB-ON-GRADE NOTES:

1. SUBGRADE BELOW SLAB-ON-GRADE SHALL BE REVIEWED AND ACCEPTED BY DIRECTOR'S REPRESENTATIVE BEFORE CONCRETE SLAB PLACEMENT.
2. THICKEN SLABS-ON-GRADE UNDER NON-BEARING MASONRY WALLS (6" THICK WALLS AND OVER) AND REINFORCE AS SHOWN ON DRAWINGS. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS.
3. CONTROL JOINT AND CONSTRUCTION JOINT LOCATIONS SHALL BE COORDINATED WITH ENGINEER OF RECORD IN ACCORDANCE WITH SPECIFICATIONS.
4. REFER TO ARCHITECTURAL DRAWINGS FOR PITCHED SLABS AND FLOOR DRAIN LOCATIONS.
5. EXTEND REINFORCEMENT IN SLAB-ON-GRADE THRU CONTROL JOINTS, TERMINATING EVERY OTHER BAR.
6. ALL SLABS-ON-GRADE SHALL BEAR ON A BASE COURSE OF CLEAN, COMPACTED CRUSHED STONE A MINIMUM OF 12" THICK. THE CRUSHED STONE SHALL BE A 50:50 MIX OF NYSDOT NO. 1 AND NO. 2 CRUSHED STONE.

TIMBER TRUSS NOTES:

1. TIMBER TRUSSES SHALL BE OF TYPE IN WHICH CHORDS AND WEB MEMBERS ARE IN ONE PLANE. USE GUSSET PLATES, WHICH DEVELOP DESIGN STRENGTH REQUIRED AT JOINTS, FOR CONNECTIONS.
2. MAXIMUM TRUSS DEFLECTION SHALL BE L/360 FOR TOTAL LIVE LOAD AND L/240 FOR TOTAL LOAD.
3. TOP AND BOTTOM CHORDS OF TRUSSES SHALL BE 2"X6" MINIMUM AND WEB MEMBERS SHALL BE 2"X4" MINIMUM. WEB MEMBERS MAY BE LOCATED BY TRUSS MANUFACTURER AS REQUIRED.
4. COMPLY WITH THE "DESIGN SPECIFICATION FOR METAL PLATE CONNECTED WOOD TRUSSES" (T.P.I.), LATEST EDITION.
5. SUBMIT SHOP DRAWINGS WHICH CONTAIN DATA IN ACCORDANCE WITH T.P.I. DRAWINGS SHALL BE SIGNED AND SEALED BY A NEW YORK STATE PROFESSIONAL ENGINEER.
6. NO SNOW LIVE LOAD REDUCTION WILL BE PERMITTED FOR SLOPED TRUSSES.
7. FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH "QUALITY CONTROL MANUAL" OF THE T.P.I.
8. CONTINUOUSLY BRACE TRUSS BOTTOM CHORDS WITH FLAT 2"X4" AT 8'-0"± O.C., LOCATED AT OR NEAR PANEL POINTS.
9. PROVIDE CONTINUOUS 2"X4" DIAGONAL CROSS BRACING NAILED TO EACH TRUSS DIAGONAL FROM TOP CHORD TO BOTTOM CHORD OF TRUSS. ONE PAIR OF DIAGONAL CROSS BRACING REQUIRED FOR EACH SIX TRUSSES.
10. PROVIDE DIAGONAL BOTTOM CHORD BRACING BETWEEN HORIZONTAL BRACING FOR FULL WIDTH OF BUILDING AT END NON-BEARING WALLS AND AT 20' O.C. THROUGHOUT LENGTH OF BUILDING.
11. PROVIDE BRACING IN ACCORDANCE WITH "COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES, HIB-91" (T.P.I.).
12. FURNISH GABLE END TRUSSES WITH VERTICAL 2"X4" STUDS AT 24" O.C. PLACE STUDS WITH STRONG AXIS PERPENDICULAR TO END WALL.
13. FIRMLY SECURE ALL BRACING TO BOTH GABLE ENDS.

CONCRETE MASONRY UNIT (CMU) WALL NOTES:

1. REFER TO THE ARCHITECTURAL DRAWINGS OR SPECIFICATIONS FOR TYPES OF MASONRY OTHER THAN CONCRETE MASONRY.
2. MORTAR SHALL CONFORM TO TABLE 1 OF ASTM C270, TYPE S. THE MORTAR MIX DESIGN (BY VOLUME) SHALL BE SUBMITTED TO THE ENGINEER BEFORE CONSTRUCTION BEGINS. HOLLOW CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 NORMALWEIGHT SPECIFICATIONS WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI. THE SPECIFIED COMPRESSIVE STRENGTH, F'M, IS 1500 PSI.
3. COARSE CONCRETE GROUT SHALL CONFORM TO TABLE 1 OF ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A SLUMP OF 8 TO 11 INCHES. GROUT MAY BE EITHER READY MIXED OR JOB MIXED. THE GROUT MIX DESIGN (BY VOLUME) SHALL BE SUBMITTED TO THE ENGINEER BEFORE CONSTRUCTION BEGINS.
4. WHEN MIXING MORTAR AND GROUT, CONTAINERS OF KNOWN VOLUME SHALL BE USED. MEASUREMENT USING SHOVELS SHALL NOT BE ALLOWED. FOR GROUT, THE SAND AND PEA GRAVEL SHALL BE TAKEN FROM SEPARATE PILES, NOT FROM A PRE-BLENDED PILE. IF MEASUREMENT BY SHOVELING OR USE OF A PRE-BLENDED PILE IS DISCOVERED, THE ENGINEER MAY REQUIRE ALL WALLS BUILT SO FAR TO BE TESTED PER ASTM C 1314 BY CUTTING 3 MASONRY PRISMS AND 3 GROUT CORES OUT OF THE WALL FOR EVERY 5,000 SQUARE FEET OF WALL, AND MAY REQUIRE ANY AREA OF WALL TESTING BELOW 1,500 PSI TO BE REPLACED AT NO COST TO THE OWNER.
6. ALL 8" LOAD BEARING CMU WALLS (IDENTIFIED ON THE ROOF PLAN) SHALL BE REINFORCED VERTICALLY WITH #5 BARS, AT 4 FEET ON CENTER. VERTICAL REINFORCEMENT SHALL EXTEND TO THE TOP OF ALL PARAPETS. PROVIDE REINFORCEMENT BARS ALL AROUND ALL OPENINGS, EXTENDING 2 FEET PAST EACH CORNER. REFER TO THE LINTEL SCHEDULE FOR ADDITIONAL REINFORCEMENT. ALL TOP COURSES SHALL HAVE HORIZONTAL REINFORCEMENT. ALL REINFORCEMENT BARS IN CMU WALLS SHALL BE PROVIDED WITH 1" CONCRETE GROUT COVER.
7. PROVIDE VERTICAL CONTROL JOINTS AT LOCATIONS APPROVED BY THE ARCHITECT, WITH A MAXIMUM SPACING OF 20 FEET. HORIZONTAL BOND BEAM REINFORCEMENT SHALL CONTINUE THROUGH ALL CONTROL JOINTS IN ALL WALLS (BOTH LOAD-BEARING AND NON-LOAD BEARING WALLS). CONTROL JOINTS SHALL CONSIST OF A VERTICAL MASONRY JOINT, RAKED BACK AND CAULKED.

ADDENDUM DRAWING 01/29/2015

NYS OFFICE OF GENERAL SERVICES
Serving New York
ANDREW M. CUOMO
Governor
ROANN M. DESTITO
Commissioner

CONSULTANT

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FORT EDWARD, NEW YORK

MICHAEL D. PANICHELLI — 091429 — LICENSED PROFESSIONAL ENGINEER — STATE OF NEW YORK

CONTRACT:
CONSTRUCTION

TITLE:
PROVIDE CENTRAL SECURITY

LOCATION:
HIGHLAND RESIDENTIAL CENTER
NORTH CHODIKEE LAKE ROAD
HIGHLAND, NY 12528

CLIENT:
OFFICE OF CHILDREN AND FAMILY SERVICES

MARK	DATE	DESCRIPTION
–	12/7/2015	BID DOCUMENTS

PROJECT NUMBER:	44854- C
DESIGNED BY:	A. DAUSMAN
DRAWN BY:	S. PUZIER
FIELD CHECK:	
APPROVED:	C. DOOLEY

SHEET TITLE:
GENERAL NOTES

DRAWING NUMBER:
S-001

SHEET 15 OF 66

ASSET_NUMBER DWG_TYPE

Jan 12,2016 - 9:15am
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